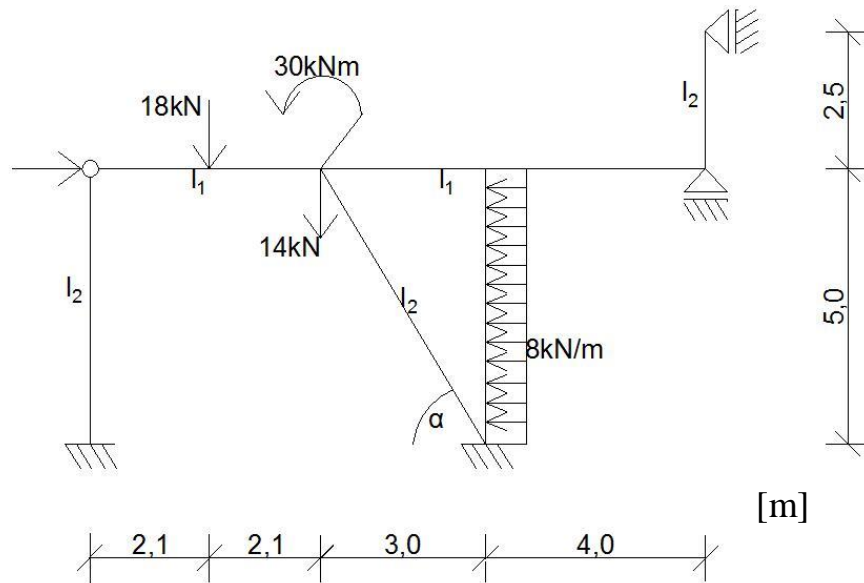


METODA PRZEMIESZCZEŃ

Wykonały:
Katarzyna Tomczyszyn
Katarzyna Winięcka

SCHEMAT KONSTRUKCJI



1. Przyjąć przekroje I1 i I2 z profili dwuteowych (IN, IPE, HEB, HEA):

Przyjęto przekroje:

$$I\ 240\ PE\ I_{y1} = 3890\ \text{cm}^4$$

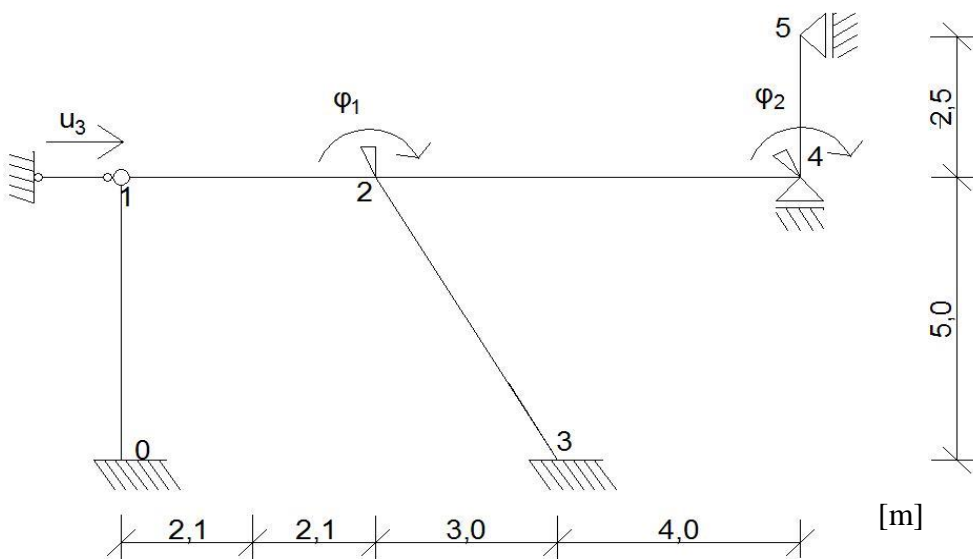
$$\underline{EI_1} = 7974,50\ \text{kNm}^2$$

$$I\ 200\ PE\ I_{y2} = 1940\ \text{cm}^4$$

$$\underline{EI_2} = 3977\ \text{kNm}^2$$

2. Korzystając z metody przemieszczeń obliczyć siły przekrojowe (M, T, N) od zadanego obciążenia oraz wykonać kontrolę kinematyczną i kontrolę statyczną:

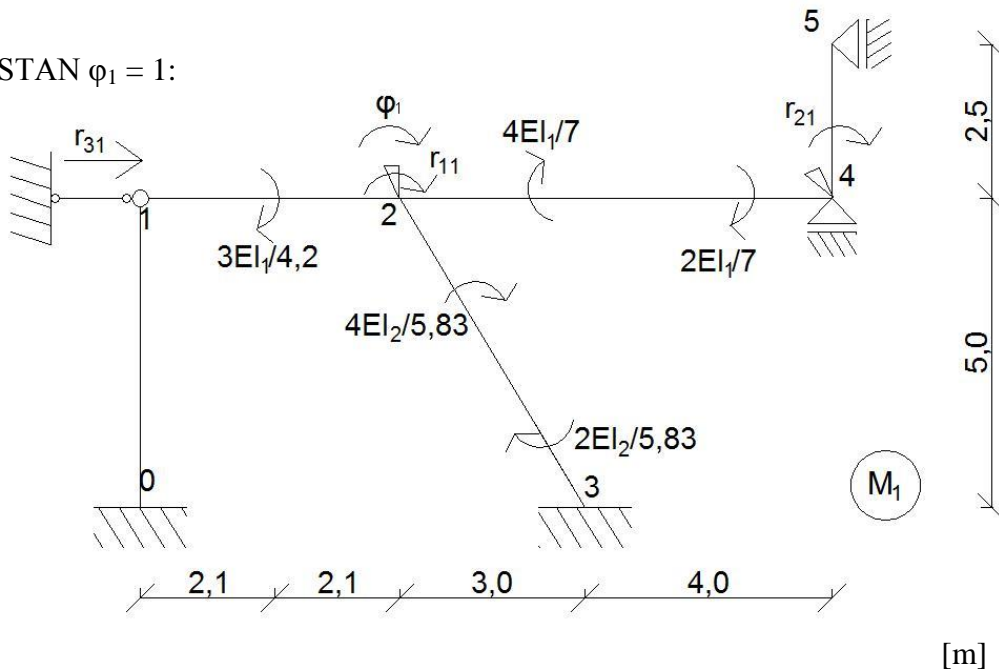
UKŁAD PODSTAWOWY: Stopień kinematycznej niewyznaczalności : **SKN = 3**



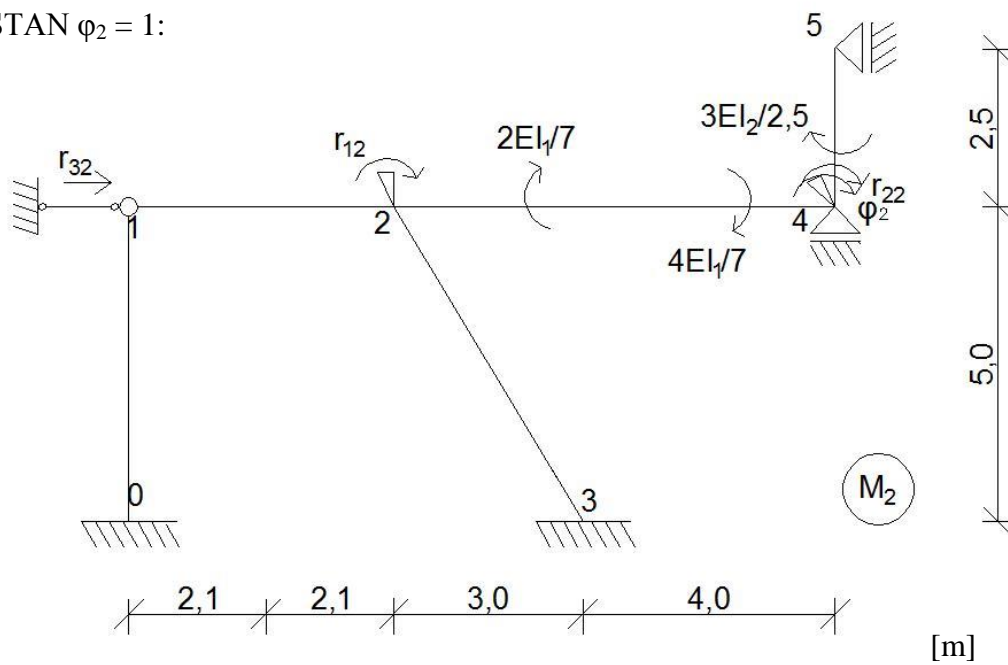
UKŁAD RÓWNAŃ KANONICZNYCH:

$$\begin{cases} r_{11} \cdot \varphi_1 + r_{12} \cdot \varphi_2 + r_{13} \cdot u_3 + r_{1p} = 0 \\ r_{21} \cdot \varphi_1 + r_{22} \cdot \varphi_2 + r_{23} \cdot u_3 + r_{2p} = 0 \\ r_{31} \cdot \varphi_1 + r_{32} \cdot \varphi_2 + r_{33} \cdot u_3 + r_{3p} = 0 \end{cases}$$

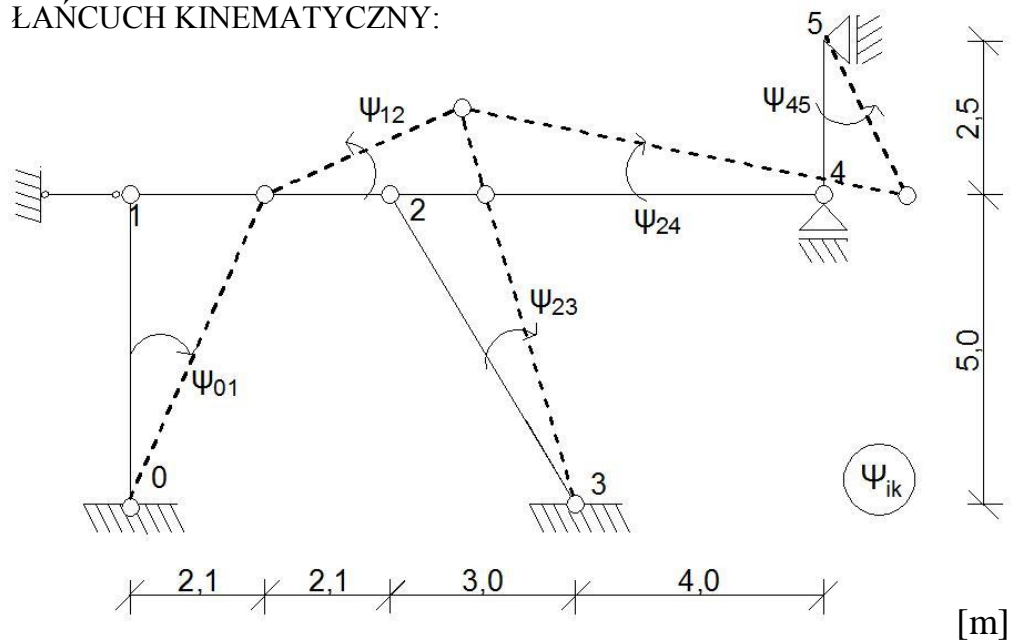
STAN $\varphi_1 = 1$:



STAN $\varphi_2 = 1$:

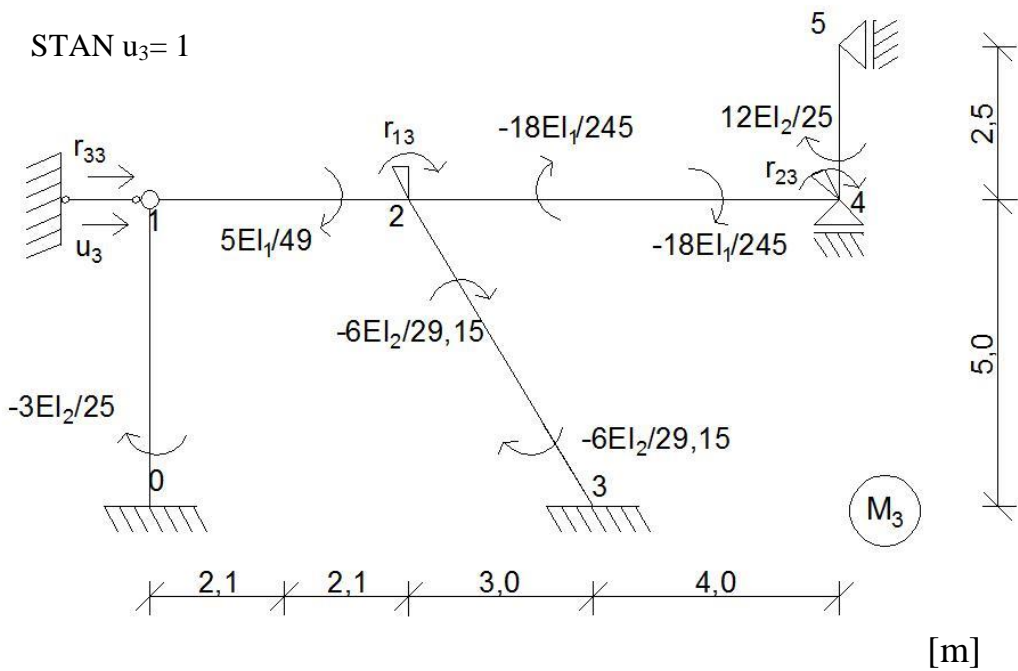


ŁAŃCUCH KINEMATYCZNY:

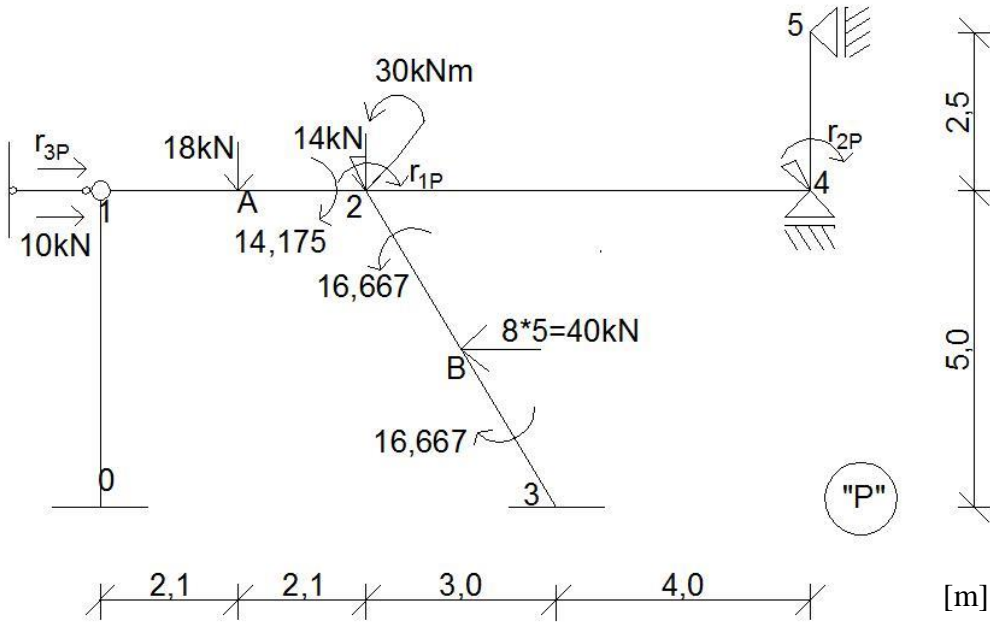
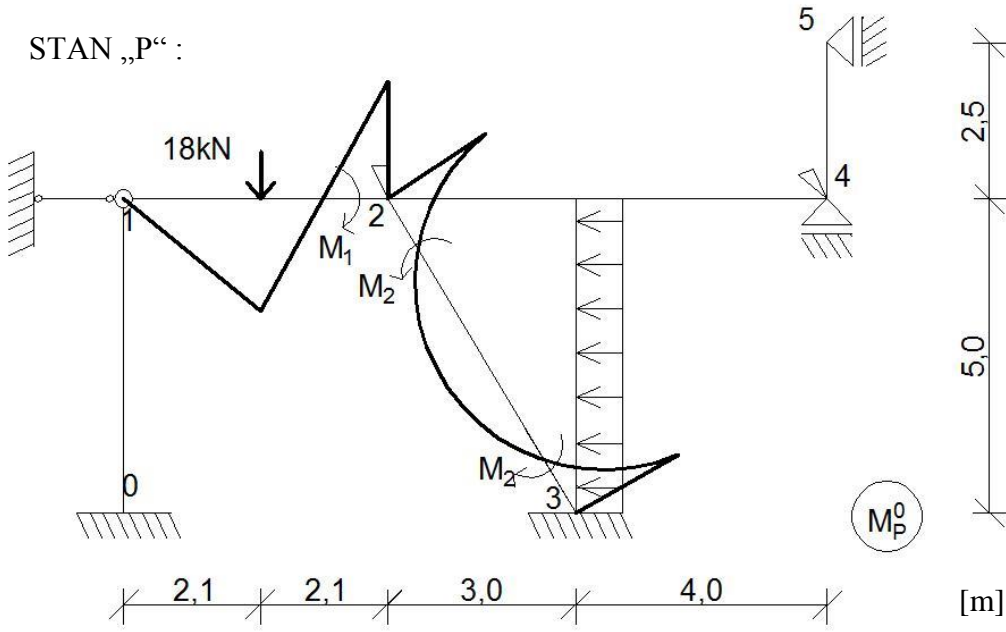


$\sin \alpha = 0,85763$
 $\cos \alpha = 0,51458$

STAN $u_3 = 1$



STAN „P”:



RÓWNANIA ŁAŃCUCHA KINEMATYCZNEGO:

$$\downarrow 01A \quad 0 + 0 + 2,1 \Psi_{12} = \delta_A^V$$

$$\delta_A^V = -0,3$$

$$\downarrow 012 \quad 0 + 0 + 4,2 \Psi_{12} = \delta_2^V$$

$$\delta_2^V = -0,6$$

$$\rightarrow 3B \quad 0 + 2,5 \Psi_{32} = \delta_B^H$$

$$\delta_B^H = 0,3$$

$$r_{11} = 1,285714 EI_1 + 0,686106 EI_2 = 12995,29198$$

$$r_{12} = r_{21} = 2278,428571$$

$$r_{31} \cdot \bar{1} + \left(\frac{4EI_2}{5,83} + \frac{2EI_2}{5,83}\right) \cdot \overline{0,2} + \frac{3EI_1}{4,2} \cdot (\overline{-0,142857}) + \left(\frac{4EI_1}{7} + \frac{2EI_1}{7}\right) \cdot (\overline{0,085714}) = 0$$

$$r_{13} = r_{31} = -0,205832 EI_2 + 0,028571 EI_1 = -590,754043$$

$$r_{12} = 0,285714 EI_1 = 2278,428571$$

$$r_{22} = 1,2 EI_2 + 0,571429 EI_1 = 9329,257143$$

$$r_{32} \cdot \bar{1} + \left(\frac{2EI_1}{7} + \frac{4EI_1}{7}\right) \cdot (\overline{0,085714}) + \frac{3EI_2}{2,5} \cdot (\overline{-0,4}) = 0$$

$$r_{23} = r_{32} = 0,48 EI_2 - 0,073469 EI_1 = 1323,08146$$

$$r_{13} = r_{31} = -0,205832 EI_2 + 0,10204 EI_1 - 0,073469 EI_1 = -590,75442$$

$$r_{23} = r_{23} = 0,48 EI_2 - 0,073469 EI_1 = 1323,08146$$

$$r_{33} \cdot \bar{1} + (-0,12 EI_2) \cdot \overline{0,2} + 0,10204 EI_1 \cdot (\overline{-0,142857}) + (-0,205832 EI_2 \cdot 2) \cdot (\overline{0,2}) + (0,48 EI_2) \cdot (\overline{-0,4}) + (-0,073469 EI_1 \cdot 2) \cdot (\overline{0,085714}) = 0$$

$$r_{33} = 1403,152507$$

$$r_{3P} \cdot \bar{1} + 10 \cdot \bar{1} + 18 \cdot \overline{-0,3} + 14 \cdot \overline{-0,6} - 40 \cdot \overline{0,5} + 14,175 \cdot \overline{-0,142857} + (16,667 - 16,667) \cdot \overline{0,2} - 30 \cdot 0 = 0$$

$$r_{3P} = 25,825 \text{ kN}$$

$$r_{2P} = 0$$

$$r_{1P} = -16,667 + 30,0 + 14,175 = 27,508 \text{ kNm}$$

ROZWIĄZANIE UKŁADU RÓWNAŃ KANONICZNYCH:

$$\begin{bmatrix} 12985,291 & 2278,315 & -590,754 \\ 2278,315 & 9329,029 & 1322,834 \\ -590,754 & 1322,834 & 1403,152 \end{bmatrix} \cdot \begin{Bmatrix} \varphi_1 \\ \varphi_2 \\ u_3 \end{Bmatrix} + \begin{Bmatrix} 27,508 \\ 0 \\ 25,825 \end{Bmatrix} = \begin{Bmatrix} 0 \\ 0 \\ 0 \end{Bmatrix}$$

$$\varphi_1 = -0,003997 \text{ rad}$$

$$\varphi_2 = 0,004419 \text{ rad}$$

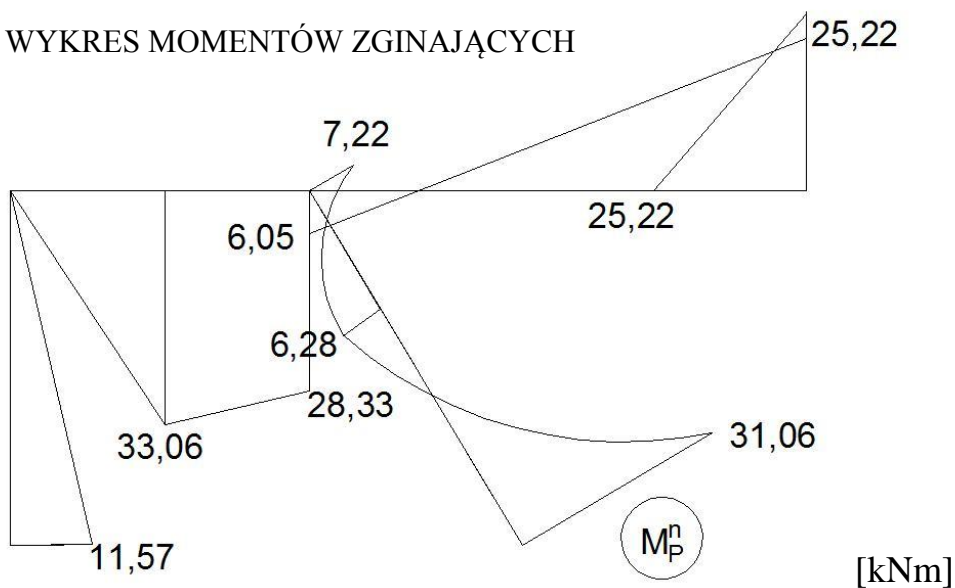
$$u_3 = -0,02425 \text{ m}$$

WYZNACZENIE M_p^n :

$$M_p^{(n)} = M_1 \cdot \varphi_1 + M_2 \cdot \varphi_2 + M_3 \cdot \Delta_3 + M_p^0$$

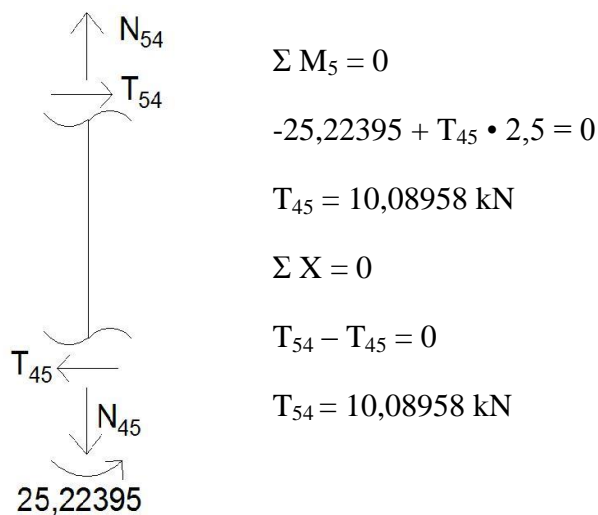
ik	M_1	M_2	M_3	M_p^0	M_{ik}^n [kNm]
01	0	0	-477,24	0	11,5734
21	5696,0714	0	813,7245	14,175	-28,32843
23	2728,6449	0	-818,5935	-16,667	-7,72299
32	1364,3225	0	-818,5935	16,667	31,064286
24	4556,8571	2278,4286	-585,8816	0	6,05104
42	2278,4286	4556,8571	-585,8816	0	25,21809
45	0	4772,4	1908,96	0	-25,22395

WYKRES MOMENTÓW ZGINAJĄCYCH



WYZNACZENIE WARTOŚCI SIŁ TNĄCYCH:

PRĘT 45:



PRĘT 42 :



$$\Sigma M_2 = 0$$

$$6,05104 + 25,21809 + T_{42} \cdot 7 = 0$$

$$T_{42} = -4,46702 \text{ kN}$$

$$\Sigma Y = 0$$

$$T_{24} - T_{42} = 0$$

$$T_{24} = -4,46702 \text{ kN}$$

PRĘT 21:



$$\Sigma M_1 = 0$$

$$-28,32843 + 18 \cdot 2,1 + T_{21} \cdot 4,2 = 0$$

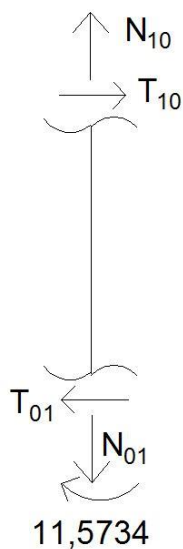
$$T_{21} = -2,25514 \text{ kN}$$

$$\Sigma Y = 0$$

$$2,25514 - 18 + T_{12} = 0$$

$$T_{12} = 15,74486 \text{ kN}$$

PRĘT 10:



$$\Sigma M_1 = 0$$

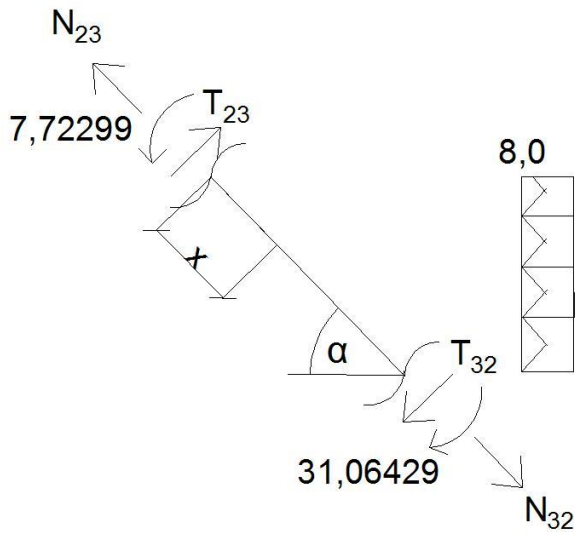
$$11,5734 + T_{01} \cdot 5 = 0$$

$$T_{01} = -2,31468 \text{ kN}$$

$$\Sigma X = 0 \quad T_{10} - T_{01} = 0$$

$$T_{10} = -2,31468 \text{ kN}$$

PRĘT 23:



$$\Sigma M_2 = 0$$

$$31,064286 - 7,72299 + 8 \cdot 5 \cdot 2,5 + T_{32} \cdot 5,83 = 0$$

$$T_{32} = -21,156312 \text{ kN}$$

$$\Sigma M_3 = 0$$

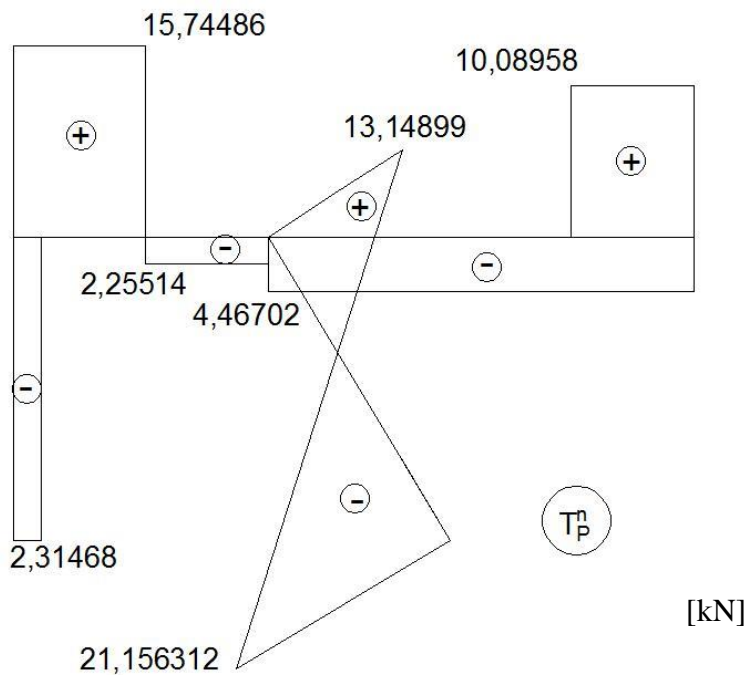
$$31,064286 - 7,72299 - 8 \cdot 5 \cdot 2,5 + T_{23} \cdot 5,83 = 0$$

$$T_{23} = 13,14899 \text{ kN}$$

$$x = \frac{T^L}{q^+} = \frac{13,14899}{8 \cdot \sin^2 \alpha} = 2,23 \text{ m}$$

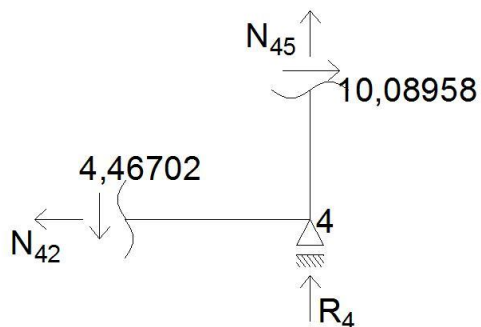
$$M_e = 6,28 \text{ kNm}$$

WYKRES SIŁ TNĄCYCH:



WYZNACZENIE WARTOŚCI SIŁ NORMALNYCH:

WĘZEL 4:



$$N_{45} = 0$$

$$\Sigma Y = 0$$

$$-4,46702 + 0 + R_4 = 0$$

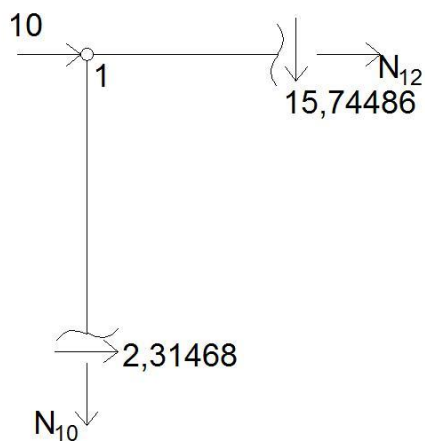
$$R_4 = 4,46702 \text{ kN}$$

$$\Sigma X = 0$$

$$N_{42} - 10,08958 = 0$$

$$N_{42} = 10,08958 \text{ kN}$$

WĘZEL 1:



$$\Sigma X = 0$$

$$10 + 2,31468 + N_{12} = 0$$

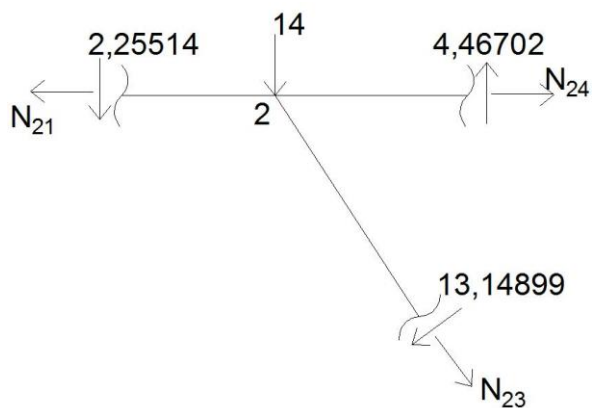
$$N_{12} = -12,31468 \text{ kN}$$

$$\Sigma Y = 0$$

$$-15,74486 - N_{10} = 0$$

$$N_{10} = -15,74486 \text{ kN}$$

WĘZEL 2:



$$N_{21} = N_{12} = -12,31468 \text{ kN}$$

$$N_{24} = N_{42} = 10,08958 \text{ kN}$$

$$\Sigma Y = 0$$

$$2,25514 + 14 - 4,46702 + N_{23} \cdot \sin \alpha + 13,14899 \cdot \cos \alpha = 0$$

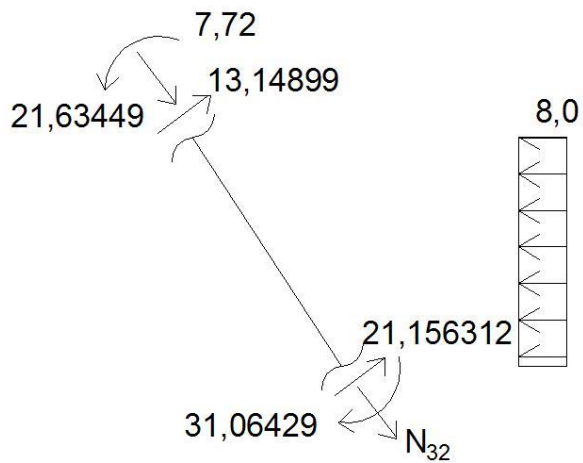
$$N_{23} = -21,634429 \text{ kN}$$

SPRAWDZENIE: $\Sigma X = 0$

$$12,31468 + 10,08958 + N_{23} \cdot \cos\alpha - 13,14899 \cdot \sin\alpha = 0$$

$$N_{23} = -21,624002 \text{ kN}$$

PRĘT 23:

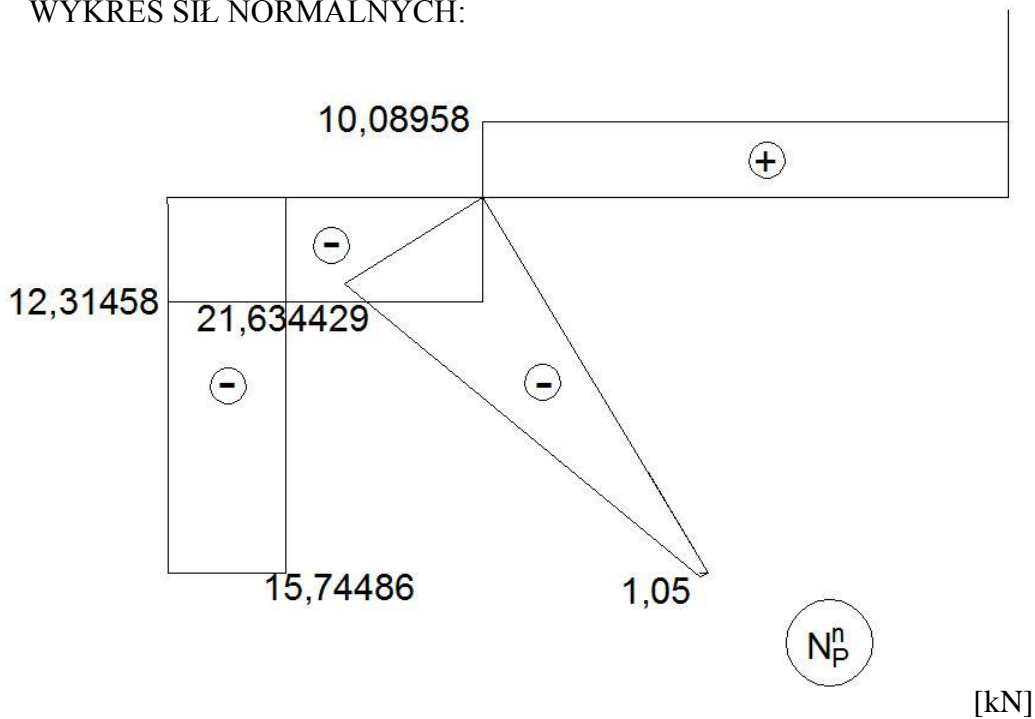


$$\Sigma I \text{ pręta} = 0$$

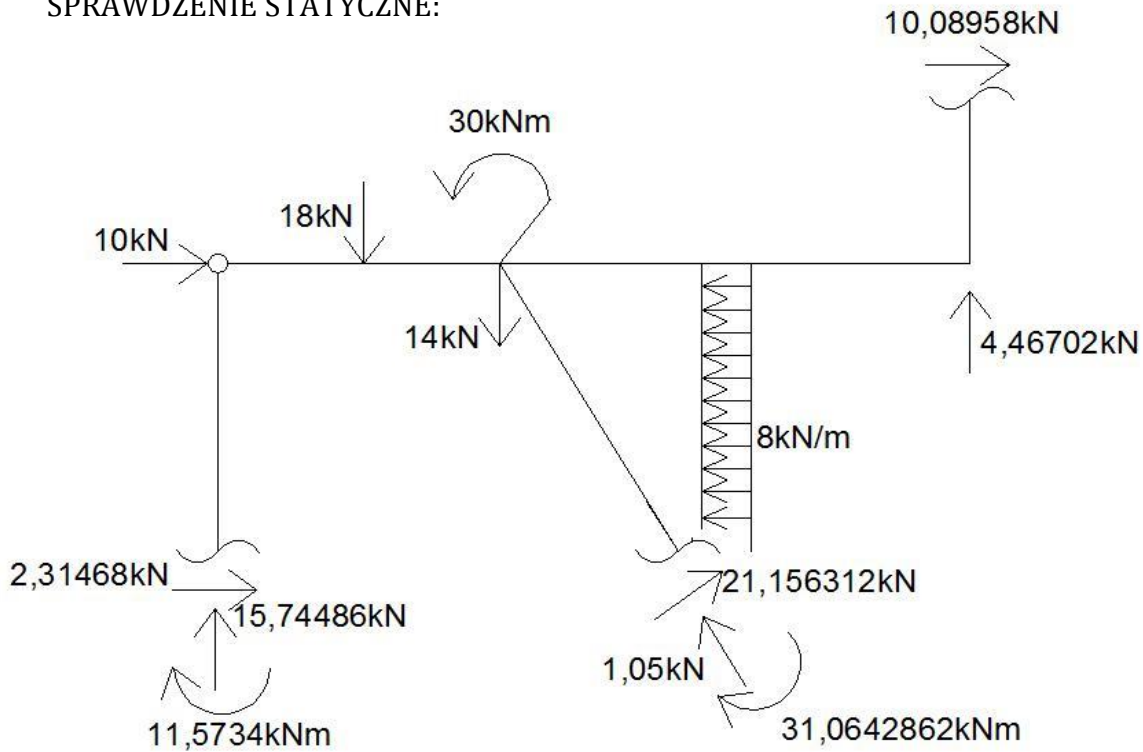
$$21,63449 + N_{32} - 8 \cdot 5 \cos\alpha = 0$$

$$N_{32} = -1,05 \text{ kN}$$

WYKRES SIŁ NORMALNYCH:



SPRAWDZENIE STATYCZNE:



$$\Sigma X = 0$$

$$2,31468 + 10 - 8 \cdot 5 + 10,08958 + 21,156312 \sin \alpha - 1,05 \cos \alpha = 0,0082 \text{ kN} \approx 0$$

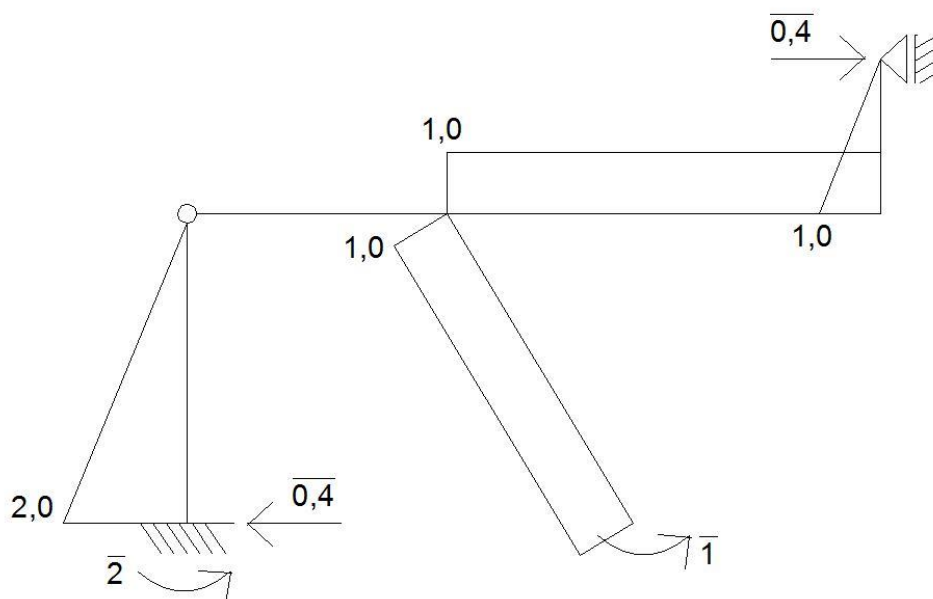
$$\Sigma Y = 0$$

$$15,74486 - 18 - 14 + 4,46702 + 21,156312 \cos \alpha + 1,05 \sin \alpha = 0,00099 \text{ kN} \approx 0$$

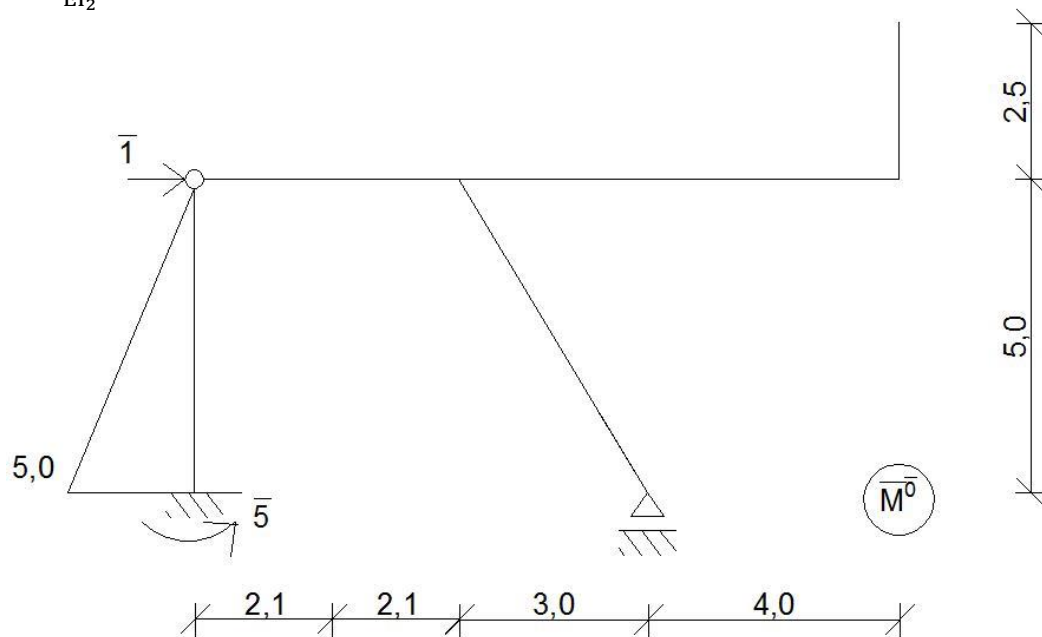
$$\Sigma M_2 = 0$$

$$15,74486 \cdot 4,2 + 11,5734 - 2,31468 \cdot 5 - 18 \cdot 2,1 - 30 + 31,0642862 - 21,156312 \cdot 5,83 + 10,08958 \cdot 2,5 - 4,46702 \cdot 7 + 8 \cdot 5 \cdot 2,5 = 0,006 \text{ kNm} \approx 0$$

SPRAWDZENIE KINEMATYCZNE:



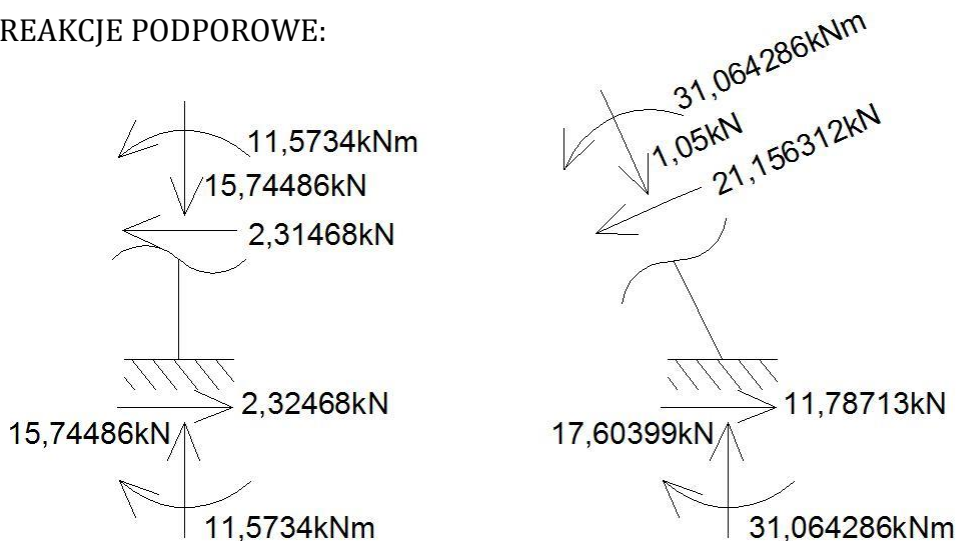
$$\bar{1} \cdot \varphi_3 = \frac{1}{EI_2} \left[-\frac{1}{2} \cdot 16,667 \cdot 5,83 \cdot 1 - \frac{1}{2} \cdot 16,667 \cdot 5,83 \cdot 1 + \frac{2}{3} \cdot \frac{8 \cdot 5^2}{8} \cdot 5,83 \cdot 1 \right] = -\frac{0,001943}{EI_2} = -4,886 \cdot 10^{-7} \text{ rad} \approx 0$$



$$\bar{1} \cdot u = \Sigma \int \frac{M_p^n \cdot \bar{M}^0}{EI} dx$$

$$\bar{1} \cdot u = \frac{1}{EI_2} \left[-\frac{1}{2} \cdot 11,57340 \cdot 5 \cdot \frac{2}{3} \cdot 5 \right] = -\frac{96,445}{EI_2} = -0,02418 \text{ m} \approx u_3$$

REAKCJE PODPOROWE:



$$\begin{cases} V_0 = 15,74486 \text{ kN} \\ H_0 = 2,314680 \text{ kN} \\ M_0 = 11,5734 \text{ kNm} \end{cases}$$

$$\begin{cases} V_3 = 11,78713 \text{ kN} \\ H_3 = 17,60399 \text{ kN} \\ M_3 = 31,064286 \text{ kNm} \end{cases}$$

SPRAWDZENIE NAPRĘŻEŃ:

$$\frac{M_{max}}{W} \leq \sigma_{dop} = 21,5 \frac{kN}{cm^2}$$

Dla przekroju I₁ maksymalny moment wynosi: 33,06 kNm

Wskaźnik wytrzymałości W dla tego przekroju wynosi: 324 cm³

$$\sigma = \frac{3306kNcm}{324cm^3} = 10,2037 \frac{kN}{cm^2} < \sigma_{dop}$$

Dla przekroju I₂ maksymalny moment wynosi: 31,06 kNm

Wskaźnik wytrzymałości W dla tego przekroju wynosi: 194 cm³

$$\sigma = \frac{3106kNcm}{194cm^3} = 16,0103 \frac{kN}{cm^2} < \sigma_{dop}$$

Oba przekroje spełniają warunek wytrzymałościowy.